

Taking the above values into consideration, most of the charts and tables have been prepared for three grades of steel having characteristic strength  $f_y$  equal to 250 N/mm<sup>2</sup>, 415 N/mm<sup>2</sup> and 500 N/mm<sup>2</sup>.

**1.2.1** If the steel being used in a design has a strength which is slightly different from the above values, the chart or table for the nearest value may be used and the area of reinforcement thus obtained be modified in proportion to the ratio of the strengths.

**1.2.2** Five values of  $f_y$  (including the value for hard-drawn steel wire fabric) have been included in the tables for singly reinforced sections.

### 1.3 STRESS-STRAIN RELATIONSHIP FOR CONCRETE

The Code permits the use of any appropriate curve for the relationship between the compressive stress and strain distribution in concrete, subject to the condition that it results in the prediction of strength in substantial agreement with test results [37.1(c) of the Code]. An acceptable stress-strain curve (see Fig. 1) given in Fig. 20 of the Code will form the basis for the design aids in this publication. The compressive strength of concrete in the structure is assumed to be  $0.67 f_{ck}$ . With a value of 1.5 for the partial safety factor  $\gamma_m$  for material strength (35.4.2.1 of the Code), the maximum compressive stress in concrete for design purpose is  $0.446 f_{ck}$  (see Fig. 1).

### 1.4 STRESS-STRAIN RELATIONSHIP FOR STEEL

The modulus of elasticity of steel,  $E_s$ , is taken as 200 000 N/mm<sup>2</sup> (4.6.2 of the Code). This value is applicable to all types of reinforcing steels.

The design yield stress (or 0.2 percent proof stress) of steel is equal to  $f_y/\gamma_m$ . With a value of 1.15 for  $\gamma_m$  (35.4.2.1 of the Code), the design yield stress  $f_{yd}$  becomes  $0.87 f_y$ . The stress-strain relationship for steel in tension and compression is assumed to be the same.

For mild steel, the stress is proportional to strain up to yield point and thereafter the strain increases at constant stress (see Fig. 2). For cold-worked bars, the stress-strain relationship given in Fig. 22 of the Code will

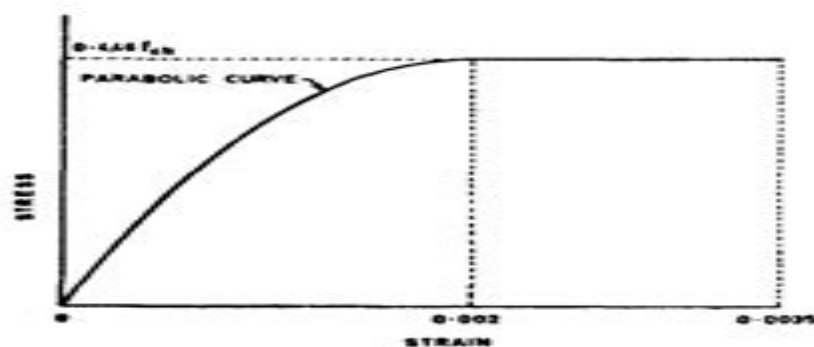


FIG. 1 DESIGN STRESS-STRAIN CURVE FOR CONCRETE

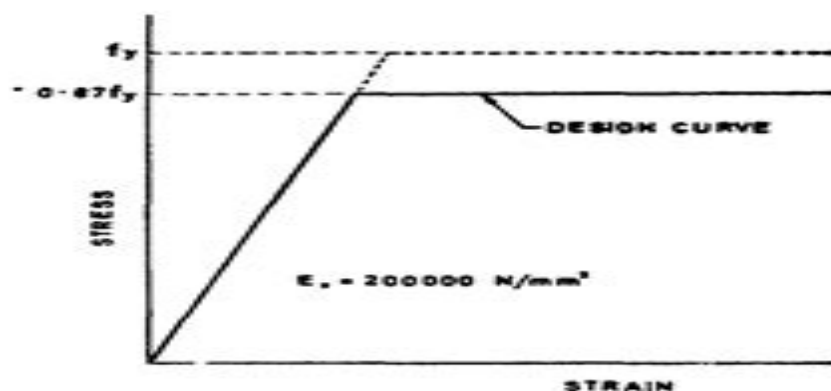


FIG. 2 STRESS-STRAIN CURVE FOR MILD STEEL

be adopted. According to this, the stress is proportional to strain up to a stress of  $0.8 f_y$ . Thereafter, the stress-strain curve is defined as given below:

| Stress      | Inelastic strain |
|-------------|------------------|
| $0.80 f_y$  | Nil              |
| $0.85 f_y$  | 0.000 1          |
| $0.90 f_y$  | 0.000 3          |
| $0.95 f_y$  | 0.000 7          |
| $0.975 f_y$ | 0.001 0          |
| $1.0 f_y$   | 0.002 0          |

The stress-strain curve for design purposes is obtained by substituting  $f_{yd}$  for  $f_y$  in the above. For two grades of cold-worked bars with 0.2 percent proof stress values of 415 N/mm<sup>2</sup> and 500 N/mm<sup>2</sup> respectively, the values of total strains and design stresses corresponding to the points defined above are given in Table A (see page 6). The stress-strain curves for these two grades of cold-worked bars have been plotted in Fig. 3.

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**Brendan G. Carr**



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