Beams on Elastic Foundations— Solution by Relaxation Methods

By W. Wright, B.Sc., Ph.D., A.M.LC.E.

Several pure mithematical marriage have been obtained for bears resting on electic foundations, sec. for entaingle, Terongle's "Theoretical fool Mechanica but those are restricted to certain standard types of loading and to constant floweral rigidity of the beam.

A practical application of these solutions is represented by a beam (usually concerte) reating on a soil which is insupped to have a constant subprade condules this similation. Desire the appearance presented to be cholest that foundation unit distance, but dimensions (472.7).

In practice, there is a considerable vertation in the value at the subgrade modulus, the assumed decimal rigidity of the boson and the assumed values of lead-This is, therefore, a type of problem particularly sained to solution by Robanston Methods, an exact polytion is unabbiainable but a solution to any required degree of accuracty in easily bound. In view of the encortainty of the other data, particularly the subgrade medicine, the degree of accuracy aimed at need not be exacting.

The relaxational carthod is readily adaptable to are type of loading and to are variation in flavoral rigidaralong the beam, whereas such variations tend to make the adultion by ours mathematics very complex and tredformer.

Сомением Единтине——

The governing equation is, from the usual beam Commence of the last

El
$$\frac{dy}{dx^2} + ky - \kappa(x) = \kappa$$

s, y are Cartesian m-ordinates Ed. — the flexunal ciridity of so the flexusal rigidity of the beam

or(x) - the lineding intensity at a

- Subgrade Medulos.

As a test of the religiation method, the "standard case " of a central point had acting on a from of unitorn flammal rigidity has been worked out and checked against the salution as obtained by the more conventhinked transference.

Replacemental, Appropriate

The beam is divided into a number of equal parts b. apart. The governing equation is then expressed in facts defineence form and values of the wanted function. y are guessed at each point in the suage. These values are saludituted in the finite difference equation for the point and the resultant error or residual framet. This is then hereistated by meane of suitable operations, the had recided being reads as small as we please, nonpatible with the accuracy required. When the definetions have been found, the practically important shours and bending moments are nouly calculated.

Francis Диропичност Алуномималися

The generaling equation is $\frac{E T d^3 y}{dx^4} + \delta y - W(x) \Rightarrow 0$.

and this must be expressed in finite difference flegs.

Considering 3 adjacent points of subdivision, the finite

difference form of
$$\frac{d^2 h^2 y}{dx^2}$$
 at the point x is $h^2 \left(-\frac{d^2 y}{dx^2} \right)_{\beta} =$

一种的变形 医心中的心 Differentiating twice and multiplying by #4

$$\frac{d^{2}}{dx^{2}}\left(\frac{dx^{2}}{dx^{2}}\right)_{2} = -\frac{2d^{2}x^{2}x^{2}}{dx^{2}} + \frac{2d^{2}x^{2}}{dx^{2}} + \frac{2d^{2}x^{2}x^{2}}{dx^{2}}$$

$$= -4(-2x_{1}+x_{1}+x_{2}) + 2x_{1} + 2x_{2} - 2x_{1} + 2x_{1} + x_{2} - 2x_{3}$$

... The equation expressed to finite difference form is

$$-\frac{dy_1+dy_2+dy_3}{dT} = u - P_3$$

$$-\frac{dy_2}{dT} = u - P_3$$

The relaxation pattern for inpublishing residuals will be ther this opposed poems as

In order to tetospect the Innuclary conditions we need

alter
$$\frac{d^2y}{dx^4}$$
 and $\frac{d^2y}{dx^2}$ expressed in faite difference form.

not Reddonest to-

$$A^{\mu} \left(\frac{d^{2}y}{dx^{2}} \right)_{\mu} = -2dy_{\mu} + 1d(y_{\mu} + y_{\mu}) = -\frac{2}{4\pi} (y_{\mu} + y_{\mu})$$

 $A^{\mu} \left(\frac{d^{2}y}{dx^{2}} \right) = 2y_{\mu} - 4y_{\mu} + 4y_{\mu} - 4y_{\mu}.$

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Scientific and Technical Aerospace Reports ,1995 Modelling of Soil-Structure Interaction V. Kolár, I. Nemec, 2012-12-02 Distributed in the East European countries China Northern Korea Cuba Vietnam and Mongolia by Academia Prague CzechoslovakiaThis book is based on the efficient subsoil model introduced by the authors in 1977 and applied in the last ten years in the design of foundations From the designer's point of view the model considerably reduces the extent of the calculations connected with the numerical analysis of soil structure interaction. The algorithms presented are geared for use on mini and personal computers and can be used in any numerical method A special chapter is devoted to the implementation of the model in the NE XX finite element program package illustrated with diagrams tables and practical examples Besides presenting the energy definition and general theory of both 2D and 3D model forms the book also deals with practical problems such as Kirchhoff's and Mindlin's foundation plates interaction between neighbouring structures actual values of physical constants of subsoils and natural frequencies and shapes of foundation plates Today researchers and engineers can choose from a wide range of soil models some fairly simple and others very elaborate However the gap which has long existed between geomechanical theory and everyday design practice still persists The present book is intended to suit the practical needs of the designer by introducing an efficient subsoil model in which the surrounding soil is substituted by certain properties of the structure soil interface When a more precise solution is required a more sophisticated model form can be used Its additional degrees of deformation freedom can better express the behaviour of layered or generally unhomogeneous subsoil As a result designers will find that this book goes some way towards bridging the above mentioned gap between structural design theory and day to day practice **Applied Mechanics Reviews** ,1973

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